

UNIT MASONRY ASSEMBLIES (SEE SPECIFICATION SECTION 042200)

- CONCRETE MASONRY UNITS (CMU) SHALL BE ERECTED AS LOAD BEARING CONCRETE MASONRY. COMPLY WITH ACI 530.1 "SPECIFICATION FOR MASONRY STRUCTURES" FOR MATERIALS, METHODS, AND WORKMANSHIP AND ERECTION TOLERANCES.
- PROVIDE CONCRETE MASONRY UNIT ASSEMBLIES (CMUS) AS INDICATED ON THE DRAWINGS THAT DEVELOPS A MINIMUM NET AREA COMPRESSIVE STRENGTH (F_N) OF **2000 PSI** AT 28 DAYS AND AS FOLLOWS:
 - CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90 WITH A MINIMUM AVERAGE NET AREA COMPRESSIVE STRENGTH OF **2000 PSI** AT 28 DAYS AND AS FOLLOWS.
 - MAXIMUM WEIGHT CLASSIFICATION: NORMAL WEIGHT, UNLESS OTHERWISE NOTED
 - SIZE: MANUFACTURED TO DIMENSIONS 3/8" LESS THAN NOMINAL DIMENSIONS
- PROVIDE MORTAR AND GROUT MATERIALS AS INDICATED IN THE SPECIFICATIONS AND CONFORMING TO THE REQUIREMENTS LISTED BELOW. ALL CELLS CONTAINING REINFORCEMENT CELLS BELOW GRADE, AND ALL LOCATIONS NOTED ON THE DRAWINGS SHALL BE GROUTED SOLID. DO NOT USE ADMIXTURES, INCLUDING AIR-ENTRAINING AGENTS, ACCELERATORS, RETARDERS, WATER-REPELLENT AGENT, ANTIFREEZE COMPOUNDS, OR OTHER ADMIXTURES UNLESS OTHERWISE NOTED. DO NOT USE CALCIUM CHLORIDE IN MORTAR OR GROUT.
- LAY SOLID MASONRY UNITS WITH COMPLETELY FILLED BED AND HEAD JOINTS. BUTTER ENDS WITH SUFFICIENT MORTAR TO FILL HEAD JOINTS AND SHOVE INTO PLACE. DO NOT DEEPLY FURROW BED JOINTS OR SLUSH HEAD JOINTS.
- PROVIDE VERTICAL REINFORCING AS NOTED PER THE CMU WALL REINFORCING SCHEDULE AND PER THE REQUIREMENTS BELOW. PROVIDE MATCHING DOWELS INTO THE FOOTING OR FOUNDATION CONSTRUCTION. PROVIDE TWO ADDITIONAL BARS AND DOWELS UNDER POINT LOADS, LINTELS AND BEAMS WHICH HAVE A REACTION EXCEEDING 10 KIPS, WHETHER OR NOT NOTED ON THE FRAMING PLANS.
 - REINFORCING STEEL SHALL BE PLACED IN COMPLIANCE WITH ACI 530.1
 - PROVIDE AN OPEN BOTTOM BOND BEAM REINFORCED WITH 2 NO. 5 CONTINUOUS BARS AT THE FOLLOWING LOCATIONS AND AS NOTED ON THE DRAWINGS:
 - AT THE TOP OF ALL WALL ELEVATIONS
 - AT ALL JOIST AND FRAMING BEARING ELEVATIONS
 - EQUALLY SPACED BETWEEN LATERAL SUPPORTS OR AT 10'-0" O.C. MAXIMUM VERTICALLY, IF THE DISTANCE BETWEEN LATERAL SUPPORTS EXCEEDS 10'-0"
 - GROUT VOIDS BETWEEN BRICK VENEER AND CMU WALLS OR CONCRETE WALLS SOLID BELOW GRADE. GROUT ALL CMU CELLS SOLID BELOW GRADE.
- VERIFY REINFORCING FOR CMU MASONRY PER THE REINFORCING SCHEDULES SHOWN ON THESE DRAWINGS. IF REINFORCEMENT IS NOT SPECIFICALLY INDICATED, VERIFY WITH THE SER.
- PROVIDE MISCELLANEOUS ANCHORS AS INDICATED AND COMPLY WITH THE FOLLOWING:
 - ANCHOR BOLTS: HEADED OR L-SHAPED STEEL BOLTS COMPLYING WITH ASTM A307, GRADE A, WITH ASTM A563 HEX NUTS AND, WHERE INDICATED, FLAT WASHERS; HOT-DIPPED GALVANIZED TO COMPLY WITH ASTM A153, CLASS C
 - POST INSTALLED ANCHORS: PROVIDE CHEMICAL ANCHORS, WITH CAPABILITY TO SUSTAIN, WITHOUT FAILURE, A LOAD EQUAL TO SIX TIMES THE LOAD IMPOSED WHEN INSTALLED IN SOLID OR GROUTED UNIT MASONRY AND EQUAL TO FOUR TIMES THE LOAD IMPOSED WHEN INSTALLED IN CONCRETE.
- PROVIDE STEEL, MASONRY, AND CONCRETE LINTELS AS NOTED ON THE LINTEL SCHEDULE.
- PROVIDE CONTROL AND EXPANSION JOINTS AS NOTED ON THE ARCHITECTURAL DRAWINGS, BUT SPACING SHALL NOT EXCEED THE LESSER OF A LENGTH-TO-HEIGHT RATIO OF 1:6 AND 25'-0" O.C. INSTALL CONTROL AND EXPANSION JOINT MATERIALS IN UNIT MASONRY AS MASONRY PROGRESSES. DO NOT ALLOW MATERIALS TO SPAN CONTROL AND EXPANSION JOINTS WITHOUT PROVISION TO ALLOW FOR IN-PLANE FLOOR OR PARTITION MOVEMENT.
- DURING CONSTRUCTION, COVER TOPS OF WALLS, PROJECTIONS, AND SILLS WITH WATERPROOF SHEETING AT END OF EACH DAY'S WORK. COVER PARTIALLY COMPLETED MASONRY WHEN CONSTRUCTION IS NOT IN PROGRESS.
- DO NOT APPLY UNIFORM FLOOR OR ROOF LOADS FOR AT LEAST 12 HOURS AND CONCENTRATED LOADS FOR AT LEAST 3 DAYS AFTER BUILDING MASONRY WALLS OR COLUMNS.
- DO NOT USE FROZEN MATERIALS OR MATERIALS MIXED OR COATED WITH ICE OR FROST. DO NOT BUILD ON FROZEN SUBSTRATES. REMOVE AND REPLACE UNIT MASONRY DAMAGED BY FROST OR BY FREEZING CONDITIONS. COMPLY WITH COLD-WEATHER CONSTRUCTION REQUIREMENTS CONTAINED IN ACI 530.1.
- COMPLY WITH HOT-WEATHER CONSTRUCTION REQUIREMENTS CONTAINED IN ACI 530.1.

STRUCTURAL SHEATHING

- PLYWOOD AND ORIENTED STRAND BOARD SHALL COMPLY TO EITHER DOC PS 1 OR DOC PS 2, UNLESS OTHERWISE INDICATED. EACH PANEL SHALL BE FACTORY MARKED TO INDICATE COMPLIANCE WITH APPLICABLE STANDARD.
 - COMPLY WITH APPLICABLE RECOMMENDATIONS IN AIA FORM NO. E308, "AIA DESIGN/CONSTRUCTION GUIDE: RESIDENTIAL & COMMERCIAL," INCLUDING "CODE PLUS INSTALLATION PROVISIONS" FOR TYPES OF STRUCTURAL-USE PANELS AND APPLICATIONS INDICATED.
- PLYWOOD OR ORIENTED STRAND BOARD WALL SHEATHING SHALL BE EXPOSURE 1, APA RATED STRUCTURAL SHEATHING CONFORMING TO THE REQUIREMENTS SHOWN ON THESE DRAWINGS.
- INTERIOR WALLS UNLESS NOTED OTHERWISE SHALL BE SHEATHED WITH PAPER-SURFACED GYPSUM WALL SHEATHING, CONFORMING TO ASTM C 79 OR ASTM C 1396. GYPSUM SHEATHING, WITH WATER-RESISTANT TREATED CORE AND WITH WATER-REPELLENT PAPER BONDED TO CORE'S FACE, BACK, AND LONG EDGES. FASTEN GYPSUM SHEATHING TO WOOD FRAMING WITH NAILS PER ICC'S "INTERNATIONAL BUILDING CODE."
- PLYWOOD OR ORIENTED STRAND BOARD ROOF SHEATHING SHALL BE EXTERIOR, APA RATED STRUCTURAL SHEATHING CONFORMING TO THE REQUIREMENTS SHOWN ON THESE DRAWINGS.
- PLYWOOD OR ORIENTED STRAND BOARD SUBFLOORING SHALL BE EXPOSURE 1, APA RATED STRUCTURAL SINGLE-FLOOR PANELS OR SHEATHING CONFORMING TO THE REQUIREMENTS SHOWN ON THESE DRAWINGS.
- SHEATHINGS SHALL BE ATTACHED PER THE REQUIREMENTS SHOWN ON THESE DRAWINGS OR AS REQUIRED PER ICC'S INTERNATIONAL BUILDING CODE, WHICHEVER IS MORE STRINGENT.
- WHEN REQUIRED, ADHESIVES FOR FIELD JOINING PANELS TO FRAMING SHALL COMPLY WITH ASTM D 308 THAT IS APPROVED FOR USE WITH TYPE OF CONSTRUCTION PANEL INDICATED BY MANUFACTURERS OF BOTH ADHESIVES AND PANELS.

STRUCTURAL STEEL (SEE SPECIFICATION SECTION 051200)

- STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE FOLLOWING STANDARDS AND THE LATEST EDITIONS OF SAID STANDARDS:
 - AISC'S "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES"
 - AISC'S "SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS" AND "SUPPLEMENT NO. 2," IF THE RESPONSE MODIFICATION FACTOR IS GREATER THAN 1.0
 - AISC'S "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS"
 - AISC'S "SPECIFICATION FOR THE DESIGN OF STEEL-HOLLOW STRUCTURAL SECTIONS"
 - RCS'S "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS"
 - AWSS'S "STRUCTURAL WELDING CODE AWS D1.1"
- STEEL FABRICATORS FOR THIS PROJECT SHALL PARTICIPATE IN THE AISC QUALITY CERTIFICATION PROGRAM AND HAVE A MINIMUM DESIGNATION OF STD. STEEL INSTALLERS FOR THIS PROJECT SHALL PARTICIPATE IN THE AISC QUALITY CERTIFICATION PROGRAM AND HAVE A MINIMUM DESIGNATION OF CSE. ALL PERSONNEL PERFORMING WELDING ON THIS PROJECT SHALL CONFORM TO THE QUALITY PROCEDURES ACCORDING TO AWS D1.1 "STRUCTURAL WELDING CODE-STEEL".
- ALL STRUCTURAL STEEL MATERIALS SHALL CONFORM TO THE FOLLOWING:
 - W-SHAPES: ASTM A992
 - CHANNELS, M: CHANNELS: ASTM A36 (A992 ACCEPTABLE)
 - ANGLES: ASTM A36
 - PLATE AND BAR: ASTM A36
 - COLD-FORMED HOLLOW STRUCTURAL SECTIONS (HSS): ASTM A500, GRADE C
 - STEEL PIPE: ASTM A53, TYPE E OR S, GRADE B
 - WELDING ELECTRODES: CLASS EXXX
- UNLESS OTHERWISE NOTED ON THE DESIGN DOCUMENTS, APPLY SHOP PRIMER PER SPECIFICATIONS TO ALL STEEL SURFACES NOT EXPOSED TO WEATHER EXCEPT THE FOLLOWING:
 - SURFACES EMBEDDED IN CONCRETE OR MORTAR. EXTEND PRIMING OF PARTIALLY EMBEDDED MEMBERS TO A DEPTH OF 2".
 - SURFACES TO BE FIELD WELDED.
 - SURFACES TO BE HIGH-STRENGTH BOLTED WITH SLIP-CRITICAL CONNECTIONS
 - SURFACES TO RECEIVE SPRAYED FIRE-RESISTIVE MATERIALS
 - GALVANIZED SURFACES
- APPLY ZINC COATING BY THE HOT-DIP PROCESS ACCORDING TO ASTM A123 TO LOOSE LINTELS, RELINING AND LINTELS, END ANGLES AND CHANNELS AT DOCK LEVELERS, AND ALL STEEL EXPOSED TO WEATHER. FILL VENT HOLES AND GRIND SMOOTH AFTER GALVANIZING AS REQUIRED. REPAIR DAMAGED GALVANIZED COATINGS WITH GALVANIZED REPAIR PAINT ACCORDING TO ASTM A370 AND MANUFACTURER'S WRITTEN INSTRUCTIONS.
- BOLTS, CONNECTORS, AND ANCHORS SHALL CONFORM TO THE FOLLOWING:
 - ASTM F3125 BOLTED CONNECTIONS:
 - GR A325, TYPE 1, HEAVY HEX STEEL STRUCTURAL BOLTS; ASTM A563 HEAVY HEX CARBON-STEEL NUTS; AND ASTM F436 HARDENED CARBON-STEEL WASHERS. THE FINISH FOR THESE BOLTED CONNECTIONS SHALL BE PLAIN UNLESS CONNECTING HOT-DIPPED GALVANIZED MATERIALS AND THEN SHALL HAVE A HOT-DIPPED ZINC COATING CONFORMING TO ASTM A153
 - OR A490, TYPE 1, HEAVY HEX STEEL STRUCTURAL BOLTS; ASTM A563 HEAVY HEX CARBON-STEEL NUTS; AND ASTM F436 HARDENED CARBON-STEEL WASHERS. PLAIN. THE FINISH FOR THESE BOLTED CONNECTIONS SHALL BE PLAIN
 - ANCHOR RODS: ASTM F1554, GRADE 36 UNLESS NOTED OTHERWISE
 - NUTS: ASTM A563
 - PLATE WASHERS: ASTM A36 CARBON STEEL, REFER TO DRAWINGS AND DETAILS FOR DIMENSIONAL REQUIREMENTS
 - WHERE F1554 GR.55 ANCHOR RODS ARE NOTED ON THE DRAWINGS, THE ANCHORS SHALL INCLUDE THE S1 WELDING SUPPLEMENT.
 - FINISH: PLAIN
- DEFORMED BAR ANCHORS (DBAR): ASTM A1064 AND MEETING REQUIREMENTS FOR TYPE "C" STUDS PER AWS D1.1, F_y70KSI. REINFORCING BARS SHALL NOT BE SUBSTITUTED FOR DEFORMED BAR ANCHORS. WELD IN ACCORDANCE WITH AWS D1.1 CH.7.
- SELECT AND COMPLETE STEEL TO STEEL CONNECTIONS USING SCHEMATIC DETAILS INDICATED WITH THE FACTORED SECTIONS SHOWN ON THESE DRAWINGS AND AISC'S "MANUAL OF STEEL CONSTRUCTION, LOAD AND RESISTANCE FACTOR DESIGN, 10th EDITION." REINFORCING CONNECTIONS FOR A MINIMUM OF 30 KIPS. THE FABRICATOR'S RESPONSIBILITIES INCLUDE USING A QUALIFIED PROFESSIONAL ENGINEER, LICENSED IN NORTH CAROLINA, TO PREPARE STRUCTURAL ANALYSIS DATA FOR STRUCTURAL STEEL CONNECTIONS. HOWEVER, ALL CONNECTIONS SHALL USE A MINIMUM OF 3/4" A325 BOLTS AND WITH THE MINIMUM NUMBER OF BOLTS AS LISTED ON THESE DRAWINGS.
- IN BOLTED CONNECTIONS, PROVIDE HIGH STRENGTH BOLTS, NUTS AND WASHERS IN BOLTED STEEL CONNECTIONS AND INSTALL SAID CONNECTORS ACCORDING TO RCS'S "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS". CUT, DRILL, OR PUNCH BOLT HOLES PERPENDICULAR TO METAL SURFACES. ALL CONNECTIONS EXCEPT LISTED BELOW, OR AS INDICATED ON THE DRAWINGS, SHALL BE SNUG TIGHTENED:
 - JOINTS THAT UTILIZE OVER-SIZED HOLES
 - JOINTS THAT CONNECT BRACING MEMBERS FOR THE LATERAL RESISTING SYSTEM
 - JOINTS THAT UTILIZE SLOTTED HOLES EXCEPT THOSE WITH APPLIED LOAD APPROXIMATELY NORMAL WITHIN 90° TO 100° TO THE DIRECTION OF THE LONG DIMENSION OF THE SLOT.
 - COLUMN SPLICES IN MULTI-STORY STRUCTURES OVER 125 FT IN HEIGHT.
- IN WELDED CONNECTIONS, COMPLY WITH AWS D1.1 FOR WELDING PROCEDURE SPECIFICATIONS, TOLERANCES, APPEARANCE, AND QUALITY OF WELDS AND FOR METHODS USED IN CORRECTING WELDING WORK. COMPLY WITH AISC MINIMUM WELDING REQUIREMENTS.
- BASE AND OR BEARING PLATES WHICH ARE SUPPORTED OVER CONCRETE OR MASONRY SHALL BE PLACED OVER 2" OF GROUT WITH A TOLERANCE OF ± 1/2" UNLESS OTHERWISE NOTED. CLEAN CONCRETE AND MASONRY SURFACES OF BOND REDUCING MATERIAL AND ROUGHEN SURFACES. SET PLATES FOR STRUCTURAL MEMBERS ON WEDGES, SHIMS, OR SETTING NUTS AS REQUIRED. TIGHTEN ANCHOR RODS AFTER MEMBER IS POSITIONED AND PLUMBED. DO NOT REMOVE WEDGES, BUT IF PROTRUDING, CUT OFF FLUSH WITH BASE PLATE. PROMPTLY PACK GROUT SOLIDLY BETWEEN BEARING SURFACES SO NO VOIDS REMAIN. GROUT SHALL CONFORM TO ASTM C1107. FACTORY PACKAGED, NONMETALLIC AGGREGATE GROUT, NONCORROSIVE, NONSTAINING, MIXED WITH WATER TO CONSISTENCY SUITABLE FOR APPLICATIONS. GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 5000 PSI.
- FURNISH ANCHORAGE ITEMS EMBEDDED OR ATTACHED TO OTHER CONSTRUCTION BY USE OF SETTING DIAGRAMS AND TEMPLATES. DO NOT FLOAT-IN THESE ITEMS.
- ACCURATELY FINISH ENDS OF COLUMNS AND OTHER MEMBERS TRANSMITTING BEARING LOADS.
- PROVIDE TEMPORARY SHORES, GUYS, BRACES, AND OTHER SUPPORTS DURING ERECTION TO KEEP STRUCTURAL STEEL SECURE, PLUMB AND IN ALIGNMENT AGAINST TEMPORARY CONSTRUCTION LOADS AND LOADS EQUAL IN INTENSITY TO DESIGN LOADS. ALSO, PROVIDE TEMPORARY SUPPORTS IN STEEL TO STEEL CONNECTIONS AND ALL OTHER LOCATIONS PER OSHA REQUIREMENTS.
- MAINTAIN ERECTION TOLERANCES OF STRUCTURAL STEEL WITHIN AISC'S "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".
- ONLY SPLICE MEMBERS WHERE INDICATED ON THE DESIGN DOCUMENTS.
- ENGAGE A QUALIFIED INDEPENDENT TESTING AND INSPECTION AGENCY TO INSPECT FIELD WELDS AND HIGH-STRENGTH BOLTED CONNECTIONS. SHOP-BOLTED CONNECTIONS WILL BE INSPECTED ACCORDING TO RCS'S "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS". FIELD WELDS WILL BE VISUALLY INSPECTED ACCORDING TO AWS D1.1, EXCEPT FULL PENETRATION WELDS SHALL ALSO BE INSPECTED PER ULTRASONIC INSPECTION PER ASTM E164.

STEEL DECK (SEE SPECIFICATION SECTION 053100)

- METAL DECKING SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE STEEL DECK INSTITUTE SPECIFICATIONS.
- WELDING OF METAL DECKING SHALL CONFORM TO AWS D1.3, "STRUCTURAL WELDING CODE-SHEET STEEL".
- METAL ROOF DECK SHALL BE TYPE B (WIDE RIB) WITH A 1.5" PROFILE DEPTH MANUFACTURED BY NUCOR CORPORATION VULCOR DIVISION UNLESS NOTED OTHERWISE. ALTERNATE MANUFACTURERS MAY BE USED AT THE CONTRACTOR'S OPTION PROVIDED THE PROPERTIES OF THE ALTERNATE MEETS OR EXCEEDS THE METAL DECK SPECIFIED BELOW.

DECK DESIGN	
TYPE	THICKNESS
A, B22	0.0295
	0.169
	0.186
	0.192
	0.20
- METAL ROOF DECK SHALL BE GALVANIZED AND SHOP-PRIMED STEEL SHEET. ASTM A653, STRUCTURAL STEEL (SS) GRADE 50, G60 ZINC COATING, CLEAN, PRETREATED, AND PRIMED WITH MANUFACTURER'S STANDARD BAKED-ON, RUST-INHIBITIVE PRIMER. COAT SHALL BE THE MANUFACTURER'S STANDARD UNLESS OTHERWISE NOTED ON THE ARCHITECTURAL PLANS.
- METAL ROOF DECK SHALL HAVE A 3-SPAN CONDITION UNLESS NOTED OTHERWISE AND HAVE NESTED SIDELAPS.
- FASTEN ROOF DECK PANELS TO STEEL SUPPORTING MEMBERS TO INTERIOR RIBS OF DECK UNITS AS INDICATED AS INDICATED ON THE DRAWINGS. FASTEN SIDELAPS OF PANELS BETWEEN SUPPORTS AS INDICATED ON THE DRAWINGS. INSTALL DECK ENDS OVER SUPPORTING FRAMING WITH A 1 1/2" MINIMUM END BEARING AND LAP JOINTS 2". FASTEN ROOF DECK PANELS TO DIAPHRAGM PERIMETER, AS STEEL MEMBERS, EDGE ANGLES, EMBED PLATES AS INDICATED ON THE DRAWINGS. AS AN ALTERNATIVE TO 5/8" PUNCH WELDS WHERE SPECIFIED, CONTRACTOR MAY USE SIMPSON STRONG-TIE No. 12 STRONG-DRIVE XL SELF-DRILLING SCREWS WHERE THE SUPPORTING STEEL THICKNESS DOES NOT EXCEED 3/8". FASTEN SIDELAPS OF PANELS BETWEEN SUPPORTS AT THE SPACING INDICATED IN THE DRAWINGS.
- PROVIDE ROOF SLUMP PANS AND SLUMP PLATES OVER OPENINGS PROVIDED IN ROOF DECK AND EITHER WELD OR MECHANICALLY FASTEN FLANGES TO TOP OF DECK. SPACE CONNECTORS NOT MORE THAN 12" APART AND AT LEAST ONE AT EACH CORNER.
- PROVIDE MISCELLANEOUS ROOF DECK ACCESSORIES NOT SPECIFICALLY NOTED ON THESE DRAWINGS AS REQUIRED TO SUBSTRATE A COMPLETE DECK INSTALLATION. THESE ACCESSORIES MAY INCLUDE RIDGE AND VALLEY PLATES, FINISH STRIPS, END CLOSURES, REINFORCING CHANNELS, AND WELD COVER PLATES AT CHANGES IN DIRECTION OF ROOF-DECK PANELS ACCORDING TO DECK MANUFACTURER'S WRITTEN INSTRUCTIONS.
- DO NOT SUPPORT SUSPENDED LOADS FROM THE METAL ROOF DECK, OTHER THAN LAY-IN ACoustICAL CEILING TILE SYSTEMS, WITHOUT PRIOR WRITTEN APPROVAL BY THE DECK MANUFACTURER AND REVIEW BY THE SER. PROVIDE A SECONDARY FRAMING SYSTEM, UNISTRUT OR SIMILAR, SPANNING BETWEEN STRUCTURAL STEEL BEAMS/JOISTS TO SUPPORT PERMANENT SUSPENDED LOADS BELOW METAL ROOF DECK.
- INSTALL DECK PANELS AND ACCESSORIES ACCORDING TO APPLICABLE SPECIFICATIONS AND COMMENTARY IN SDI PUBLICATION No. 30, MANUFACTURER'S WRITTEN INSTRUCTIONS AND REQUIREMENTS IN THESE DOCUMENTS.
- INSTALL TEMPORARY SHORING BEFORE PLACING DECK PANELS, IF REQUIRED TO MEET DEFLECTION LIMITATIONS.
- PLACE DECK PANELS ON SUPPORTING FRAME AND ADJUST TO FINAL POSITION WITH ENDS ACCURATELY ALIGNED AND BEARING ON SUPPORTING FRAME BEFORE BEING PERMANENTLY FASTENED. DO NOT STRETCH OR CONTRACT SIDELAP INTERLOCKS.
- DO NOT EMBED ELECTRICAL CONDUIT OR OTHER UTILITIES IN SLAB ON METAL DECK UNLESS NOTED OTHERWISE OR UNLESS REVIEWED AND APPROVED BY ENGINEER OF RECORD.

TILT-UP WALL PANELS (SEE SPECIFICATION SECTION 034713)

- REFER TO THE CONCRETE AND STRUCTURAL STEEL SECTIONS FOR ADDITIONAL REQUIREMENTS INCLUDING BUT NOT LIMITED TO MATERIALS REQUIREMENTS, GOVERNING SPECIFICATIONS AND COLD AND HOT WEATHER CONCRETE REQUIREMENTS.
- REFER TO ARCHITECTURAL DRAWINGS FOR CHAMBERS, REVEALS AND ANY ADDITIONAL EMBEDDED ITEMS. REFER TO ARCHITECTURAL FOR FINISHING REQUIREMENTS.
- PROVIDE 3/4" CHAMBERS ON ALL EXPOSED WALL CORNERS UNLESS INDICATED OTHERWISE ON ARCHITECTURAL DRAWINGS.
- THE CONTRACTOR IS RESPONSIBLE FOR ADDITIONAL REINFORCING NECESSARY FOR LIFTING AND ERECTION. LIFTING HARDWARE AND ALL TEMPORARY BRACING, THE PANEL DESIGNS AND DIMENSIONS, AND THE ERECTION, MIN COMPRESSIVE STRENGTH AT TIME OF LIFTING SHALL BE IN ACCORDANCE WITH LIFTING DESIGN REQUIREMENTS.
- CASTING SLAB SHALL BE CURED. SAW CUTS, CRACKS, JOINTS OR DEFECTS IN THE CASTING BED SHALL BE FILLED SO AS TO MINIMIZE TRANSFER OF THE JOINT LINE TO THE PANEL FACE.
- CONTRACTOR SHALL BE RESPONSIBLE FOR COMPATIBILITY OF CURING AGENTS, SEALANTS, AND RELEASING AGENTS UTILIZED IN THE WORK. IF PANELS ARE TO BE STACKED, THE TROWELED SURFACE SHALL BE CONSIDERED THE CASTING BED AND SHALL BE TREATED AS THE SAME.
- COLD JOINTS SHALL NOT BE PERMITTED IN AN INDIVIDUAL SITE CAST TILT-UP PANEL.
- USE ERECTION EQUIPMENT THAT WILL PREVENT DAMAGE TO EXISTING CONSTRUCTION, PERMANENT FLOOR SLABS AND PANELS. DAMAGE TO WORK SHALL BE REPAIRED OR REPLACED AT THE CONTRACTOR'S EXPENSE AND IN A MANNER ACCEPTABLE TO THE ARCHITECT PRIOR TO PAINTING OR COATING.
- PANELS NOT ATTACHED TO THE BUILDING FRAME AT THE TIME OF ERECTION SHALL BE BRACED IN POSITION USING A BRACING SYSTEM DESIGNED TO RESIST WIND, SOIL AND OTHER LOADS THAT MAY REASONABLY BE DETERMINED UNTIL STRUCTURAL CONNECTIONS HAVE BEEN MADE INCLUDING CONCRETE POUR BACK STRIP PLACEMENT ACHIEVING A MINIMUM STRENGTH OF 3.000PSI. THERE SHALL BE A MINIMUM OF TWO BRACES PER PANEL. ENGINEERING OF BRACING SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. PANEL BRACING CONNECTION SHALL BE MAINTAINED DAILY TO ASSURE TIGHTNESS.
- TEMPORARY PANEL BRACING SHALL NOT BE REMOVED UNTIL ROOF DIAPHRAGM IS COMPLETELY WELDED AND INSTALLED AND SHALL NOT RELY ON ANY MOMENT RESISTANCE PROVIDED BY THE FOUNDATION SYSTEM. CONTRACTOR TO PROVIDE FOOTINGS AS REQUIRED FOR ANCHORAGE OF TILT WALL ERECTION BRACING.
- PROVIDE CONTINUOUS BEARING UNDER ALL PANELS USING 5,000 PSI (MIN) NON-SHRINK FLOWABLE GROUT OR DRYPACK. ALL PANELS SHOULD BE GROUTED IMMEDIATELY AFTER PANELS ARE SET.
- PANELS SHALL BE SHIMMED AT A MAXIMUM OF 5'-0" ON CENTER TO UNIFORMLY LOAD THE FOUNDATION FOOTING. FAILURE TO SHIM TO THIS STANDARD MAY DAMAGE THE FOUNDATION SYSTEM.
- DRY-PACK GROUT INSTALLATION AND PREPARATION FOR WELD POCKETS AND OTHER PANEL BLOCK OUTS NOT CAST IN DURING POURING SHALL BE PERFORMED AS FOLLOWS:
 - REMOVE LAITANCE DOWN TO SOUND CONCRETE
 - SURFACE TO RECEIVE GROUT SHALL BE ROUGH AND REASONABLY LEVEL
 - SURFACE SHALL HAVE BEEN PROPERLY WET CURED
 - DO NOT USE CURING COMPOUNDS
 - CLEAN SURFACE OF OIL, GREASE, DIRT AND LOOSE PARTICLES
 - REMOVE FREE WATER FROM CONCRETE AND BOLT HOLES IMMEDIATELY BEFORE GROUTING.
- PATCH HOLES IN PANEL SURFACES CAUSED BY LIFTING AND BRACING DEVICES AS PER SPECIFICATIONS.
- REFER TO ARCHITECTURAL FOR PANEL JOINT CAULK REQUIREMENTS.
- WAIT A MINIMUM OF 28 DAYS FROM PANEL CASTING BEFORE MAKING PANEL-TO-PANEL WELDS.

STEEL JOISTS (SEE SPECIFICATION SECTION 052100)

- STEEL JOISTS SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE STEEL JOIST INSTITUTE SPECIFICATIONS.
- PROVIDE JOISTS CAPABLE OF WITHSTANDING THE UNIFORM DESIGN LOADS INDICATED. FOR JOISTS WITH NONUNIFORM LOADS PROVIDE KCS JOISTS AS NOTED ON THE FRAMING PLANS OR SPECIAL JOISTS (SP) WITH THE LOADING DIAGRAMS. IF APPLICABLE, JOISTS SHALL HAVE VERTICAL DEFLECTION LIMITS AS FOLLOWS UNLESS NOTED OTHERWISE:
 - FLOOR MEMBERS
 - 1/80 OF THE SPAN FOR LIVE LOAD
 - 1/240 OF THE SPAN FOR LIVE LOAD PLUS DEAD LOAD
 - ROOF MEMBERS
 - 1/240 OF THE SPAN FOR LIVE LOAD
 - 1/180 OF THE SPAN FOR LIVE LOAD PLUS DEAD LOAD
- PROVIDE JOISTS ACCESSORIES AND MISCELLANEOUS MATERIALS CONFORMING TO THE FOLLOWING UNLESS NOTED OTHERWISE:
 - STEEL BEARING PLATES: ASTM A36
 - CARBON STEEL BOLTS AND THREADED FASTENERS: ASTM A307, GRADE A, CARBON STEEL, HEX-HEAD BOLTS AND THREADED FASTENERS, CARBON STEEL NUTS, AND FLAT, UNHARDENED WASHERS, PLAIN FINISH
 - WELDING ELECTRODES: COMPLY WITH AWS STANDARDS
- ALL JOISTS SHALL BE SHOP PRIMED WITH SSPC-15 PAINT OR THE MANUFACTURER'S STANDARD SHOP PRIMER COMPLYING WITH THE PERFORMANCE REQUIREMENTS IN SSPC-PAINT 15, CONFORM COLORS WITH THE ARCHITECTURAL DRAWINGS. DO NOT PAINT OR PRIME WHEN JOISTS ARE TO RECEIVE SPRAYED ON FIRE-PROOFING.
- PROVIDE K-SERIES AND KCS-TYPE K-SERIES STEEL JOISTS AS INDICATED ON THE FRAMING PLANS. JOISTS SHALL BE MANUFACTURED PER SJI'S "SPECIFICATIONS". JOISTS SHALL HAVE UNDERSLING ENDS AND A PARALLEL TOP CHORD, UNLESS OTHERWISE NOTED.
- EXTEND TOP CHORDS OF JOISTS AS NOTED ON THE FRAMING PLANS. PROVIDE TYPE S EXTENSIONS, COMPLYING WITH SJI'S "SPECIFICATIONS" UNLESS NOTED OTHERWISE.
- CAMBER JOISTS ACCORDING SJI'S "SPECIFICATIONS" UNLESS NOTED OTHERWISE.
- EQUIP BEARING ENDS OF JOISTS WITH MANUFACTURER'S STANDARD BEVELED ENDS OR SLOPED SHOES IF SLOPE EXCEEDS 1/4 INCH PER 12 INCHES.
- DO NOT INSTALL JOISTS UNTIL SUPPORTING CONSTRUCTION IS IN PLACE AND SECURED. INSTALL JOISTS AND ACCESSORIES PLUMB, SQUARE, AND TRUE TO LINE. SECURELY FASTEN TO SUPPORTING CONSTRUCTION ACCORDING TO SJI'S "SPECIFICATIONS". JOIST MANUFACTURERS' WRITTEN RECOMMENDATIONS AND THE REQUIREMENTS BELOW.
 - BEFORE INSTALLATION, SPLICE JOISTS DELIVERED TO PROJECT SITE IN MORE THAN ONE PIECE.
 - SPACE, ADJUST, AND ALIGN JOISTS ACCURATELY IN LOCATION BEFORE PERMANENTLY FASTENING.
 - INSTALL TEMPORARY BRACING AND ERECTION BRIDGING, CONNECTIONS, AND ANCHORS TO ENSURE JOISTS ARE STABILIZED DURING CONSTRUCTION.
 - DELAY RIGIDLY CONNECTING BOTTOM CHORD EXTENSIONS TO COLUMNS OR SUPPORTS UNTIL DEAD LOADS HAVE BEEN APPLIED.
- FIELD WELD JOISTS TO SUPPORTING STEEL BEARING PLATES AND FRAMEWORK. COORDINATE WELDING SEQUENCE AND PROCEDURE WITH PLACEMENT OF JOISTS. COMPLY WITH AWS REQUIREMENTS AND PREVENT THE WELD FROM BEING APPEARANCE AND QUALITY OF WELDS, AND METHODS USED IN CORRECTING WELDING WORK. JOISTS SHALL BE WELDED TO THEIR SUPPORTS WITH 1/2" FILLET WELD FOR EACH SIDE OF JOIST UNLESS OTHERWISE NOTED.
- INSTALL AND CONNECT BRIDGING ACCORDING TO SJI'S "SPECIFICATIONS". BRIDGING SHALL BE INSTALLED CONCURRENTLY WITH JOIST ERECTION, BEFORE CONSTRUCTION LOADS ARE APPLIED. ANCHOR ENDS OF BRIDGING LINE AT TOP AND BOTH CHORDS IF STABILIZING AT WALLS OR BEAMS. FURNISH ADDITIONAL ERECTION BRIDGING IF REQUIRED FOR STABILITY.
- JOIST MANUFACTURER MUST CHECK THE JOIST SYSTEM FOR AN UP/LIFT PRESSURE AS NOTED ON THE UP/LIFT DIAGRAM AND PROVIDE BRIDGING AS REQUIRED TO ADEQUATELY BRACE THE BOTTOM CHORD DURING LATERAL MOVEMENT.
- FOLLOW THE TESTING AND INSPECTION REQUIREMENTS IN THE "STRUCTURAL STEEL" SECTION OF THESE SPECIFICATIONS.
- COORDINATE JOIST WEB AND BRIDGING LAYOUT WITH THE ESFR FIRE SUPPRESSION SYSTEM PRIOR TO FABRICATION.
- PROVIDE SEALED CALCULATIONS FOR SPECIAL CONDITIONS NOTED IN THE DELEGATED DESIGN SECTION OF THESE NOTES, AS REQUIRED.

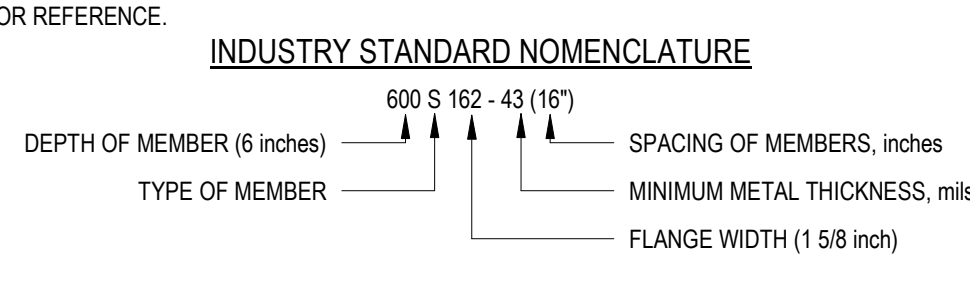
CONCRETE FLOOR AND SLABS (SEE SPECIFICATION SECTION 033500 & 033511)

- REQUIREMENTS NOTED IN THIS SECTION APPLY TO CONCRETE SLABS ON GRADE. REFER TO THE CONCRETE SECTION AND THE CONCRETE FINISHING SECTION OF THE SPECIFICATIONS FOR FURTHER REQUIREMENTS.
- CONCRETE SLABS ON GRADE SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI 302.1R "GUIDE FOR CONCRETE SLAB AND SLAB CONSTRUCTION".
- SLABS ON GRADE DEPEND ON THE INTEGRITY OF BOTH THE SLAB AND FULL SOIL SUPPORT. PROVIDE SATISFACTORY SOIL MATERIALS UNDER SLABS ON GRADE ACCORDING TO GEOTECHNICAL ENGINEER'S WRITTEN RECOMMENDATIONS. PROOF-ROLL SUBGRADE BELOW THE BUILDING SLABS WITH HEAVY PNEUMATIC-TIRED EQUIPMENT TO IDENTIFY SOFT POCKETS AND AREAS OF EXCESS YIELDING.
- COMPACT SOIL MATERIALS AND SUBGRADE TO NOT LESS THAN 98% OF MAXIMUM DRY UNIT WEIGHT, UNLESS OTHERWISE RECOMMENDED BY THE GEOTECHNICAL ENGINEER.
- PROVIDE PLASTIC VAPOR RETARDER AS SHOWN ON THE "TYPICAL SLAB ON GRADE DETAIL" ON THESE DRAWINGS. VAPOR RETARDER SHALL BE 10 MIL STEGO WRAP OR APPROVED EQUAL AND SHALL CONFORM TO ASTM E 1745, CLASS A, NOT LESS THAN 10 MILS THICK.
- PROVIDE A MINIMUM OF 4" OF GRANULAR FILL DIRECTLY UNDER SLAB ON GRADES. FILL SHALL CONSIST OF A CLEAN MIXTURE OF CRUSHED STONE OR CRUSHED OR UNCRUSHED GRAVEL PER ASTM D448, SIZE 57, WITH 100 PERCENT PASSING A 1-1/2" SIEVE, 15 TO 55 PERCENT PASSING A NO. 4 SIEVE AND LESS THAN 12 PERCENT PASSING A NO. 200 SIEVE. MATERIAL PASSING THE NO. 200 SIEVE SHOULD BE CLEAN GRANULAR FILL WITH LESS THAN 3 PERCENT CLAY OR FRABLE PARTICLES.
- PROVIDE STEEL FIBER REINFORCEMENT IN CONCRETE SLABS ON GRADE TO CONTROL CRACKING DUE TO DRYING SHRINKAGE AND THERMAL EXPANSION CONTRACTION. LOWERED WATER MIGRATION, INCREASED IMPACT CAPACITY, ABRASION RESISTANCE AND RESIDUAL STRENGTH. REFER TO SPECIFICATIONS FOR ADDITIONAL FIBER REINFORCEMENT REQUIREMENTS.
- THE CONCRETE SLAB ON GRADE HAS BEEN DESIGNED USING A SUBGRADE MODULUS PER THE FOUNDATION SPECIFICATION AND A DESIGN LOADING AS NOTED IN THE "DESIGN LOADS" SECTION OF THESE SPECIFICATIONS. THE SER IS NOT RESPONSIBLE FOR DIFFERENTIAL SETTLEMENT, SLAB CRACKING OR OTHER FAILURE DEFECTS RESULTING FROM UNREPORTED CONDITIONS MITIGATING THE ABOVE ASSUMPTIONS.

COLD FORMED STEEL FRAMING (SEE SPECIFICATION SECTION 054000)

- LIGHT GAGE STRUCTURAL STEEL FRAMING SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH AISI'S "STANDARD FOR COLD-FORMED STEEL FRAMING - GENERAL PROVISIONS".
- LIGHT GAGE STRUCTURAL STEEL FRAMING AS SHOWN ON THESE PLANS CONFORMS TO THE INDUSTRY STANDARD NOMENCLATURE. EXPLANATION OF THIS NOMENCLATURE IS SHOWN BELOW FOR REFERENCE.

INDUSTRY STANDARD NOMENCLATURE



A. THE FIRST SET OF NUMBERS REPRESENTS THE DEPTH OF THE MEMBER TO TWO (2) DECIMAL PLACES WITHOUT THE USE OF A DECIMAL POINT, I.E. 160 = 16" 00, 362 = 3 6/8", ETC.

B. THE LETTER REPRESENTS THE TYPE OF MEMBER. THERE ARE ONLY FIVE LETTERS USED, S = "C" MEMBERS, T = TRACK, JS = JAMSTUD MEMBERS BY THE STEEL NETWORK, F = FURRING CHANNEL, AND U = "U" CHANNEL.

C. THE SECOND SET OF NUMBERS REPRESENTS THE FLANGE WIDTH OF THE MEMBER TO TWO (2) DECIMAL PLACES WITHOUT THE USE OF A DECIMAL POINT, I.E. 162 = 1 5/8", 200 = 2", 125 = 1 1/4", ETC.

D. THE THIRD NUMBER REPRESENTS THE MINIMUM UNCOATED METAL THICKNESS EXPRESSED IN MILS. (0.001 in.). THESE CORRESPOND TO REFERENCE GAGES AS FOLLOWS: -18 (24ga.), -27 (22ga.), -33 (20ga.), -43 (18ga.), -54 (16ga.), -68 (14ga.), -80 (13ga.), AND -97 (12ga.). THE MILS DEFINE THE MINIMUM ALLOWABLE UNCOATED METAL THICKNESS AND ARE 5% LESS THAN THE DESIGN THICKNESS. THE 5% VARIANCE IN METAL THICKNESS IS PERMITTED PER SECTION 3.4 OF THE AISI SPECIFICATION.

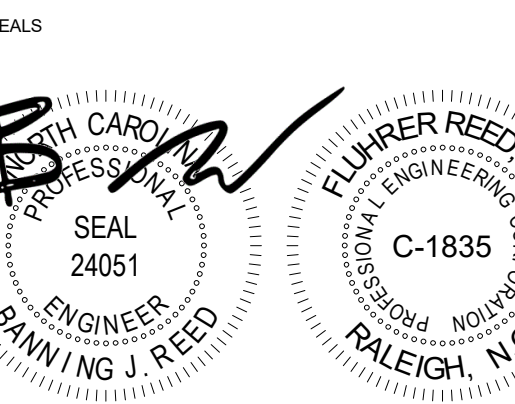
E. THE LAST NUMBER DENOTED IN (B), IS NOT PART OF THE INDUSTRY STANDARD NOMENCLATURE. HOWEVER FLUHRER REED, PA USES THIS NUMBER TO REPRESENT THE MAXIMUM SPACING OF THE MEMBERS IN THE SYSTEM.
- PREPARATION OF SHOP DRAWINGS, DESIGN CALCULATIONS, AND OTHER STRUCTURAL DATA BY A QUALIFIED PROFESSIONAL ENGINEER AS PER SPECIFICATIONS.
- PROVIDE COLD FORMED METAL FRAMING AS FOLLOWS:
 - STEEL SHEET PER SPECIFICATIONS
 - STRUCTURAL MEMBERS SHALL HAVE THE MINIMUM YIELD STRESS AS FOLLOWS:
 - 33 mil, 43 mil = 33 KSI
 - 54 mil, 68 mil, 97 mil = 50 KSI
- INSTALL COLD-FORMED METAL FRAMING AND ACCESSORIES PLUMB, SQUARE, AND TRUE TO LINE, AND WITH CONNECTIONS SECURELY FASTENED.
- LOCATE MECHANICAL FASTENERS AND INSTALL ACCORDING TO SHOP DRAWINGS, AND COMPLYING WITH REQUIREMENTS FOR SPACING, EDGE DISTANCES, AND SCREW PENETRATION AS NOTED ON THESE DRAWINGS.
- COMPLY WITH THE FOLLOWING FOR EXTERIOR NON-LOAD-BEARING WALL INSTALLATION.
 - INSTALL CONTINUOUS TRACKS SIZED TO MATCH STUDS. ALIGN TRACKS ACCURATELY AND SECURELY ANCHOR AT CORNERS AND ENDS, AND AT SPACINGS AS INDICATED.
 - FASTEN BOTH FLANGES OF STUDS TO BOTTOM TRACK, UNLESS OTHERWISE INDICATED. SPACE REQUIREMENTS AND PREVENT THE WALL FROM BEING APPEARANCE AND QUALITY OF WELDS, AND METHODS USED IN CORRECTING WELDING WORK. JOISTS SHALL BE WELDED TO THEIR SUPPORTS WITH 1/2" FILLET WELD FOR EACH SIDE OF JOIST UNLESS OTHERWISE NOTED.
 - ISOLATE NON-LOAD-BEARING STEEL FRAMING FROM BUILDING STRUCTURE TO PREVENT TRANSFER OF VERTICAL LOADS WHILE PROVIDING LATERAL SUPPORT. CLIPS SHALL ALLOW FOR AN UPWARD AND DOWNWARD MOVEMENT OF 1" UNLESS OTHERWISE NOTED.
 - WALLS UNLESS NOTED OTHERWISE SHALL BE SHEATHED WITH PAPER-SURFACED GYPSUM WALL SHEATHING, CONFORMING TO ASTM C 79 OR ASTM C 1396. GYPSUM SHEATHING, WITH WATER-RESISTANT TREATED CORE AND WITH WATER-REPELLENT PAPER BONDED TO CORE'S FACE, BACK, AND LONG EDGES. FASTEN GYPSUM SHEATHING TO STUDS PER ICC'S "INTERNATIONAL BUILDING CODE."
 - INSTALL HORIZONTAL BRIDGING IN STUD SYSTEM, SPACED AS INDICATED ON SHOP DRAWINGS BUT NOT GREATER THAN 48" O.C. FASTEN AT EACH STUD INTERSECTION.
- COMPLY WITH THE FOLLOWING FOR JOIST INSTALLATION.
 - INSTALL PERIMETER JOIST TRACK SIZED TO MATCH JOISTS, ALIGN AND SECURELY ANCHOR OR FASTEN TRACK TO SUPPORTING STRUCTURE AT CORNERS, ENDS, AND SPACINGS INDICATED.
 - INSTALL JOISTS BEARING ON SUPPORTING FRAME OR WALL, LEVEL, STRAIGHT, AND PLUMB. ADJUST TO FINAL POSITION, BRACE, AND REINFORCE. FASTEN JOISTS TO BOTH FLANGES OF JOIST TRACK, REINFORCE ENDS AND BEARING POINTS OF JOISTS WITH WEB STIFFENERS, END CLIPS, JOIST HANGERS, STEEL CLIP ANGLES, OR STEEL-STD SECTIONS AS INDICATED ON SHOP DRAWINGS.
 - PLACE JOISTS NOT MORE THAN 2 INCHES FROM ABUTTING WALLS, AND AS NOTED IN THE WALL FRAMING SCHEDULE.
 - FRAME OPENINGS WITH BUILT-UP JOIST HEADERS CONSISTING OF JOIST AND JOIST TRACK, NESTING JOISTS OR ANY OTHER COMBINATION OF CONNECTED JOISTS IF INDICATED.
 - INSTALL JOIST REINFORCEMENT AT INTERIOR SUPPORTS WITH A SINGLE, SHORT LENGTH OF JOIST SECTION LOCATED DIRECTLY OVER INTERIOR SUPPORT, WITH LAPPED JOISTS OF EQUAL LENGTH TO JOIST REINFORCEMENT, OR AS INDICATED ON THE DRAWINGS. ALSO PROVIDE WEB STIFFENERS TO TRANSFER AXIAL LOADS OF WALLS ABOVE.
 - INSTALL BRIDGING AT INTERVALS INDICATED ON SHOP DRAWINGS. FASTEN BRIDGING AT EACH JOIST INTERSECTION.
 - SECURE JOISTS TO LOAD-BEARING INTERIOR WALLS OR BEAMS TO PREVENT LATERAL MOVEMENT OF BOTTOM FLANGE.

ANCHORAGE OF ROOFTOP EQUIPMENT

- THE CONTRACTOR IS RESPONSIBLE FOR ALL COSTS RELATED TO THE DESIGN, PURCHASE, AND INSTALLATION OF NECESSARY SUPPORTS, BRACING, AND ANCHORAGE OF ROOFTOP EQUIPMENT AND COMPONENTS.
- ANCHORAGE OF CERTAIN ITEMS MAY BE SPECIFICALLY NOTED/SHOWN ON THESE STRUCTURAL DESIGN DOCUMENTS. HOWEVER, ALL ROOFTOP EQUIPMENT MUST BE ANCHORED. THE ANCHORAGE OF ALL ROOFTOP EQUIPMENT AND COMPONENTS TO THE BUILDING SUPERSTRUCTURE OR DUNNAGE NOT SHOWN ON THE STRUCTURAL DESIGN DOCUMENTS, INCLUDING BUT NOT LIMITED TO FANS, HOODS, STACKS, CURBS, CURB MOUNTED ROOF SCREENS, EQUIPMENT-MOUNTED ROOF SCREENS, SOLAR PANELS, DUCT WORK, DISPOUSERS, THE DOWNS, AND VIBRATION ISOLATORS, ARE TO BE DESIGNED BY THE CONTRACTOR'S LICENSED AND QUALIFIED SPECIALTY ENGINEER.
- THE SPECIALTY ENGINEER SHALL CONSIDER WIND AND SEISMIC LOADING AS DICTATED BY THE GOVERNING BUILDING CODES LISTED IN THE SPECIFICATIONS IN THEIR DESIGNS.
- SIGNED AND SEALED DRAWINGS AND CALCULATIONS ARE TO BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW AND APPROVAL, BEFORE CONSTRUCTION BEGINS (SEE NOTE 5). THE EQUIPMENT MANUFACTURER SHALL PROVIDE THE ATTACHMENT OF THE UNIT TO THE STRUCTURE AND SUBMIT TO THE ENGINEER LOADS, LOCATIONS, AND METHODS OF ATTACHMENT.
- THE ENGINEER OF RECORD HAS MADE PROVISIONS IN THE DESIGN OF THE PRIMARY STRUCTURAL FRAME TO ACCOMMODATE THE LOADS AND ATTACHMENTS OF THE ROOFTOP EQUIPMENT. HOWEVER, FOLLOWING THE ENGINEER OF RECORD'S REVIEW OF THE SPECIALTY ENGINEER'S ANCHORAGE DESIGN, MODIFICATIONS AND/OR REINFORCEMENTS TO THE STRUCTURE MAY STILL BE REQUIRED.
- ANCHORAGES SHALL CONSIDER THE EFFECTS OF, BUT NOT LIMITED TO, THE BASE MATERIAL THICKNESS, EDGE DISTANCES, SPACINGS AND ANCHOR EMBEDMENT DEPTH.
- REFER TO THE "SUBMITTALS" AND "DELEGATED DESIGN" SECTIONS FOR ADDITIONAL REQUIREMENTS.

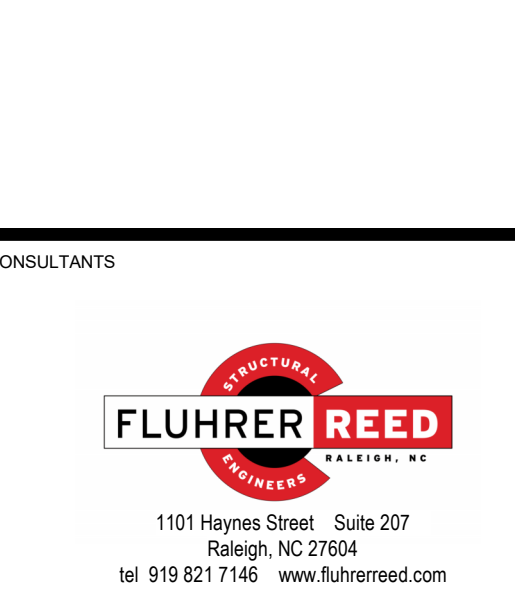


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ROOMS TO GO - PHASE II EXPANSION DUNN, NC



CLIENT



DUNN, NC 28334
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GENERAL NOTES

REVISIONS		
NUMBER	DESCRIPTION	DATE
2	BID ADDENDUM #03	02/22/24

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CHECKED BY **HMB**
DATE ISSUED **12/18/23** OF